

ABQ06

# CALCULATION COVER SHEET



<b>Project:</b>	INEEL V-Tank Remediation Project				<b>Number of Sheets:</b> 1 of 5
<b>Site:</b>	INEEL Test Area North, Idaho Falls, Idaho				
<b>Calculation Number:</b>	ABQ06 – CE002	<b>Work Order Number:</b>	12393.002.001		
<b>Subject:</b>	Excavation – Estimated Excavated Soil Volume/Storage Requirements				
<b>Rev #:</b>	<b>Date:</b>	<b>Revision:</b>	<b>Calculated by:</b>	<b>Checked by:</b>	<b>Approved:</b>
RAA		60%	Rob Ederer		
RAB	5/31/01	90%	Rob Ederer	Berg Keshian	Berg Keshian
RAC	6/27/01	90% Polish	Berg Keshian	D. Brennecke	B. Keshian
RAD	9/27/01	Draft Final	Berg Keshian	D. Brennecke	<i>[Signature]</i> 9/27/01

CLIENT/SUBJECT \_\_\_\_\_ W.O. NO. \_\_\_\_\_

TASK DESCRIPTION \_\_\_\_\_ TASK NO. \_\_\_\_\_

PREPARED BY B. KESHAV DEPT \_\_\_\_\_ DATE 5/31/01

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

APPROVED BY
DEPT _____ DATE _____

**Problem Statement:**

Estimate the volume of excavated soil, soil container capacity, and quantity of containers.

**Method of Solution:**

1. Estimate the weight of soil and compare to weight capacity of the bags.
2. Estimate the volume of soil to be excavated.
3. Estimate the number of bags required.

**Assumptions:**

Average dry soil density is 95.5 pcf.

Average total density is 111 pcf.

Soil bag capacity is 258 ~~pcf~~ cubic ft. @ 10.2% moisture content

**Sources of Formulas and References:**

INEEL-BDW Engineering Data – Soils test results (see Calculation ABQ05 – CE001)

LiftBag Product Literature

Design Drawings

**Calculation:**

1. Estimate volume of Soil for TANK Excavation

Depth: 15' Area:  $48 \times 26' = 1248 \text{ SF}$

$$\text{Vol} = 1248 \times 15 / 27 = 693 \text{ CY}$$

Exclude volume of Tanks to Spring Line

$$\frac{\frac{\pi D^3}{4} \times L}{27} \times \frac{3}{2} = \frac{\frac{3.14 \times 10^2 \times 19.5}{4}}{27} \times \frac{3}{2} = 85 \text{ CY}$$

Volume of Soil =  $693 - 85 = 608 \text{ CY}$  ←

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DEPT _____	DATE _____

2. Determine Volume of Soil for U9 & Valve Box excavation

Assume depth of 12' depth

$$\begin{aligned} Vol &= 18' \text{ wide} \times 12' \text{ Long} \times 12' \text{ deep} / 27 \\ &= 96 \text{ cy} \end{aligned}$$

Exclude U9 TANK and valve box

U9 tank is 400 gal

$$400 \text{ gal} / 7.45 \text{ gal/cy} = 53.69 \text{ CF} = 2 \text{ cy}$$

Valve box

$$6' \times 6.5' \times 8' / 27 = 11.5 \text{ cy}$$

$$Vol \text{ Soil} = 96 - 2 - 11.5 = \boxed{82.5 \text{ cy}} \leftarrow$$

3. Determine Volume of Soil from Pipe Removal

Assume lines 1' deep and trenches 3' wide

Assume <sup>200</sup>~~100~~ of lines <sup>156</sup>~~100~~ ft

$$\sqrt{200} \times 1' \times 3' / 27 = \boxed{11.1 \text{ cy}} \leftarrow$$

Neglect volume of pipe

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4) TOTAL Volume of Soil to be handled

$$608 + 82.5 + \overset{156}{\cancel{77.8}} =$$

$$\begin{array}{r} 768 \text{ cy} \\ 847 \end{array}$$

 ←

Assume 20% Expansion

$$\overset{847}{\cancel{768}} \times 1.2 =$$

$$\begin{array}{r} 922 \text{ cy} \\ 1016 \end{array}$$

 ⇐

5) Estimate number of bags

Each bag has volume of 258 cf and 2400 lb limit

Based on unit weight of Soil = 111 pcf

$$\text{Capacity of Bag} = \frac{\text{Vol bag}}{\text{Bag Vol}} = \frac{2400 \text{ lbs} / 111}{258} = 216.2 \text{ cf} = 84\%$$

$$216 \text{ cf} / 258 = 8 \text{ cy} / \text{bags}$$

$$\frac{1016}{922} \text{ cy} / 8 = \boxed{\frac{127}{115} \text{ bags of Soil}} \leftarrow$$

6. Estimate No of bags for piping

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Assume Volume of pipe is 80 cf and that pipe takes up 10 times the space of its Volume

$$10 \times 80 = 800 \text{ cf} / 27 = 29.63 \text{ cy}$$

Assume each bag can only be filled to 4.5 cy

$\therefore 29.63 / 4.5 = 6.6 \text{ bags} -$  Assume 7 bags of pipe ←

Total Bags = 122 bags ←  
134/b

7. Determine Storage Area for Bags

Assume Stack 2 high

Area of EACH bag = 52.4 sf

$\frac{134}{2} \times 52.4 / \text{bag} = 3511 \text{ sf}$

Size Area of Rectangle

$\frac{45}{40'} \times 80' = 3600 \text{ sf}$  ←

\* SPACE AVAILABLE  
5300 SF which  
Allows for bag expansion  
←

### Discussion

TOTAL Soil Volume 1016  
TOTAL Bags 122 bags  
SOIL CONTAINER CAPACITY 8 CY/BAG  $\Rightarrow$  84% CAPACITY  
Storage Area Required  $40 \times 80' = 3200 \text{ SF}$   
3600 SF ←

CLIENT/SUBJECT U TANKS W.O. NO. \_\_\_\_\_

TASK DESCRIPTION \_\_\_\_\_ TASK NO. \_\_\_\_\_

PREPARED BY B. KESHIAN DEPT \_\_\_\_\_ DATE 9/27/01

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

APPROVED BY	
DEPT _____	DATE _____

\* There is the potential that gravel Road Base within the AOC and Laydown Areas could become contaminated and require bagging for disposal

### Drum Storage

$$1' \times 3000 \text{ SF} \times \frac{1}{27} = 111 \text{ CY}$$

### HK Storage

$$\frac{1'}{27} \times (53 \times 53) + \left( \frac{53+15}{2} \times 45 \right) = 161 \text{ CY}$$

### Off Site Access Rd

$$\begin{aligned} \frac{1}{2} \times 12 \times 113 \times \frac{1}{27} &= 25 \text{ CY} \\ + \frac{1}{2} \times 20 \times 255 \times \frac{1}{27} &= 94 \text{ CY} \end{aligned}$$

### Other Gravel Areas

$$\left[ \frac{1}{2} \times 15 \times 703 \right] + \left( \frac{1}{2} \times 23 \times 105 \right) + \left( \frac{1}{2} \times 23 \times 45 \right) \times \frac{1}{27} = 83 \text{ CY}$$

TOTAL = 474 CY or  
Approx 59 Bags

ABQ07

# CALCULATION COVER SHEET



<b>Project:</b>	INEEL V-Tank Remediation Project				<b>Number of Sheets:</b> 1 of 9
<b>Site:</b>	INEEL Test Area North, Idaho Falls, Idaho				
<b>Calculation Number:</b>	ABQ07 – CE003		<b>Work Order Number:</b>	12393.002.001	
<b>Subject:</b>	Lifting Requirements				
<b>Rev #:</b>	<b>Date:</b>	<b>Revision:</b>	<b>Calculated by:</b>	<b>Checked by:</b>	<b>Approved:</b>
RAA	4/11/01	60%	R. Ederer	N/A	N/A
RAB	6/1/01	90%	R. Ederer	D. Bennecke	B.Keshian
RAC	6/27/01	90% Polish	D. Brennecke	B. Keshian	B. Keshian
RAD	9/27/01	Draft Final	D. Brennecke	B. Keshian	<i>[Signature]</i> 9/27/01

Client/Subject: INEEL-BBWI / V-tank Removal Project W.O. No.:12393.002.001

Task Description: Lifting Requirements

Task: 90% Design Calc

Prepared by: R. Ederer

### Problem Statement:

Evaluate the lifting requirements of the V-1, 2, 3, and V-9 tanks. Consider lifting of shipping casks, concrete rad-vault, and soil bags.

### Method of Solution:

1. Estimate the tank surface area.
2. Estimate thickness of the steel.
3. Estimate the tank weight.

**Assumptions:** / DATA Provided by INEEL

1. The tanks are stainless steel (type 304 for this analysis)
2. The V-1, 2, 3 tanks are 10 ft. diameter and 19.5 ft. long.
3. The V-9 tanks is 3.5 ft diameter by 5.5 ft to top of cone section, cone is 1.75' long.

### Sources of Formulas and References:

- Grove Hydraulic Crane Product literature
- Design Drawings
- Duratek shipping container and vault literature
- "Lift Liner" system product literature

### Calculation:

#### A. Calculate the expected weight of V-1, V-2, and V-3 Tanks

1. Determine the surface area of the tanks.

$$\begin{aligned} \text{Dia}_{\text{tank}} &:= 10 & \text{L}_{\text{tank}} &:= 19.5 \\ \text{Area}_{\text{surface}} &:= \left( \text{Dia}_{\text{tank}} \cdot \pi \cdot \text{L}_{\text{tank}} \right) + 2 \cdot \pi \cdot \frac{\text{Dia}_{\text{tank}}^2}{4} & 785 \text{ sf} \end{aligned}$$

$$\text{Assume the tank thickness: } t_{\text{shell}} := \frac{0.25}{12}$$

2. Determine the tank weight

$$\text{Steel volume is: } \text{Vol}_{\text{sst}} := t_{\text{shell}} \cdot \text{Area}_{\text{surface}} \quad \text{Vol}_{\text{sst}} \quad 16.35 \quad \text{cf}$$

Assume the density of Type 304 stainless steel is:

$$\text{sg}_{\text{sst}} := 8.04 \quad \rho_{\text{sst}} := \text{sg}_{\text{sst}} \cdot 62.4 \quad \rho_{\text{sst}} \quad 501.7 \quad \text{pcf}$$

The tank weight is:

$$W_{\text{tank}} := \rho_{\text{sst}} \cdot \text{Vol}_{\text{sst}} \quad W_{\text{tank}} = 8203 \quad \text{lbs}$$

Assume approximately 2" of liquid or sludge left in the tank when lifted.

$$s := \frac{2}{12} \quad r := 5 \quad \alpha := \frac{s}{r} \quad \alpha = 0.033$$

$$W_{\text{liquid}} := (0.5 \cdot r^2 \cdot \alpha - \sin(\alpha)) \cdot L_{\text{tank}} \cdot (62.4 \cdot 1.2) \quad W = 601 \quad \text{lbs}$$

*in Adjust* Adjust tank weight for fittings, flanges, and miscellaneous piping (1,000 lbs).

$$W_{\text{tank}} := (W_{\text{tank}} + W_{\text{liquid}} + 1000) \quad W_{\text{tank}} = 9804 \quad \text{lbs}$$

**B. Estimate the weight of the V-9 tank.** To be conservative and simplify the calculation, assume the cone section is a cylinder.

$$\text{Dia}_{\text{tank.v9}} := 4 \quad L_{\text{tank.v9}} := 5 + 1.5$$

$$\text{Area}_{\text{surface.v9}} := \left( \text{Dia}_{\text{tank.v9}}^2 \cdot \frac{\pi}{2} \right) + L \cdot (\pi \cdot \text{Dia}_{\text{tank.v9}}) \quad \text{area} := 101.71 \quad \text{sf}$$

$$\text{Assume the tank thickness:} \quad t_{\text{shell}} := \frac{0.25}{12}$$

2. Determine the tank weight

$$\text{Steel volume is:} \quad \text{Vol}_{\text{sst.v9}} := t_{\text{shell}} \cdot \text{Area}_{\text{surface.v9}} \quad \text{Vol}_{\text{sst.v9}} = 2.11 \quad \text{cf}$$

Assume the density of Type 304 stainless steel is:

$$\text{sg}_{\text{sst}} := 8.04 \quad \rho_{\text{sst}} := \text{sg}_{\text{sst}} \cdot 62.4 \quad \rho_{\text{sst}} = 501.7 \quad \text{pcf}$$

The tank weight is:

$$W_{\text{tank.v9}} := \rho_{\text{sst}} \cdot \text{Vol}_{\text{sst.v9}} \quad W_{\text{tank.v9}} = 1063 \quad \text{lbs}$$

Adjust tank weight for fittings, flanges, and miscellaneous piping (500 lbs).

$$W_{\text{tank.v9}} := W_{\text{tank.v9}} + 500 \quad 1563 \quad \text{lbs}$$

4059

**C. Evaluate the lifting requirements for the tanks. Design lifting for V-1, 2, & 3 tanks**

1. In summary, the weight of each tanks is:

$$V-1, V-2, \text{ and } V-3 \text{ (each): } W_{\text{tank}} = 9804 \text{ lbs}$$

$$V-9: W_{\text{tank.v9}} = 1563 \text{ lbs}$$

2. Assume the tanks are placed in granular fill material (cohesion = 0), with no groundwater therefore neglect the suction required to overcome the capillary forces.

$$c := 0 \quad c_a := c \cdot 0.9$$

3. Add the force to overcome soil friction. Assume the soil rises to the springline of the tank (5').

a. Determine the active soil force on the tank

$$\gamma_{\text{soil}} := 111 \text{ pcf} \quad H := 5 \quad \phi := 32 \cdot \text{deg}$$

$$K_a := \frac{1 - \sin(\phi)}{1 + \sin(\phi)} \quad K_a = 0.307$$

$$P_{\text{vertical}} := \gamma_{\text{soil}} \cdot H = 555$$

$$P_{\text{active}} := K_a \cdot P_{\text{vertical}} \quad P_{\text{active}} = 172 \text{ psf}$$

b. Determine the skin friction coefficient on the tank.

$$\delta := 17 \cdot \text{deg} \quad Ca := 0.9 \quad f_o := c_a + P_{\text{active}} \cdot \tan(\delta) \quad f_o = 53.5 \text{ psf}$$

c. Estimate the surface area of the tank in contact with soil.

$$LTD \quad \frac{(19.5 \times 10)}{2} \quad s_{\text{tank}} = 306 \text{ sf}$$

d. Estimate the breaking force to overcome the soil friction

$$P_{\text{break}} := f_o \cdot s_{\text{tank}} \quad P_{\text{break}} = 16,387 \text{ lbs}$$

4. Estimate the lifting requirement for removing the largest tank.

$$\text{Lift}_{\text{capacity}} := (W_{\text{tank}} + P_{\text{break}}) \quad \text{Lift}_{\text{capacity}} = 26191 \text{ lbs or } 13.1 \text{ tons}$$

$$9804 + 16387$$

## Discussion

Summary of calcs:

weight of each V-1, V-2, and V-3 tanks (tons):      W ea.tank=4.9 tons

weight of V-9 tank (tons):      W<sub>tank.v9</sub>      0.781 tons

Tank lifting capacity required (tons)      Lift<sub>capacity</sub>      13.1 tons

Select a Grove RT650E ~~Series~~ Rough Terrain Hydraulic Crane or equal. A cut sheet is included as Attachment 1. This crane will be adequate for other lifting requirements on the job such as the "Rad Vault 8-120" with loaded HIC (66,700 lbs + 7,500 lbs = 74,200 lbs), DURATEK CNS 8-120B Type B shipping casks (49,300 lbs, empty; 63,980 lbs w/maximum payload) and soil bags (24,000 lbs).

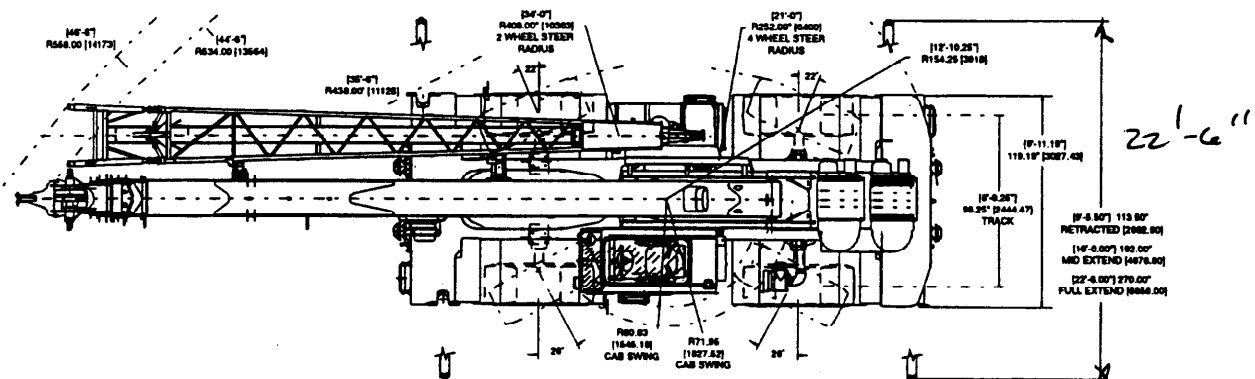
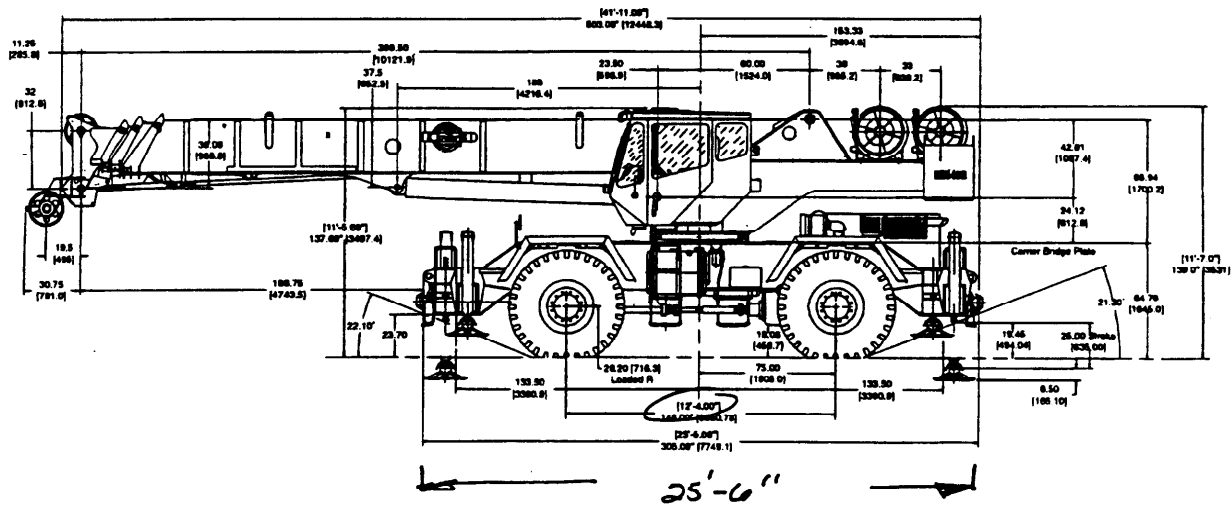
# GROVE

## RT600E SERIES



### ROUGH TERRAIN HYDRAULIC CRANE

# Dimensions



Note: ( ) Reference dimensions in mm

**RT650E RATED LIFTING CAPACITIES IN POUNDS**  
**33 FT. - 105 FT. BOOM**  
**ON OUTRIGGERS FULLY EXTENDED - 360°**

Radius in Feet	#0001								
	Main Boom Length in Feet								
	33	40	50	60	70	80	90	100	105
10	100,000 (69.5)	80,550 (73.5)	67,250 (77)						
12	87,100 (65.5)	79,150 (70.5)	64,200 (75)	*56,100 (78)					
15	69,050 (59.5)	69,550 (65.5)	59,950 (71)	51,800 (75)	45,200 (77.5)				
20	50,500 (47.5)	50,950 (57)	51,400 (64.5)	44,500 (69.5)	38,550 (73)	34,450 (75.5)	*31,400 (78)		
25	38,300 (32)	38,850 (47)	39,350 (58)	39,650 (64.5)	37,100 (68.5)	29,850 (72)	27,250 (74.5)	21,000 (76.5)	18,350 (77.5)
30		30,700 (34.5)	31,200 (50.5)	31,500 (58.5)	31,700 (64)	26,350 (68)	24,100 (71)	21,000 (73.5)	18,350 (74.5)
35			25,450 (41.5)	25,750 (52.5)	26,000 (59)	23,650 (64)	21,500 (67.5)	19,150 (70)	18,350 (71.5)
40			20,850 (30.5)	21,200 (46)	21,600 (54)	21,350 (59.5)	19,400 (64)	16,650 (67)	17,300 (68.5)
45				17,100 (38)	17,350 (48.5)	17,300 (55)	17,300 (60)	14,650 (64)	15,750 (65.5)
50				13,950 (28)	14,150 (42.5)	14,200 (50.5)	14,200 (56)	13,000 (60.5)	14,300 (62.5)
55					11,700 (35)	11,750 (45.5)	11,850 (52)	11,900 (57)	12,000 (59)
60					9,730 (26)	9,870 (39.5)	9,980 (47.5)	10,100 (53.5)	10,150 (55.5)
65						8,300 (33)	8,440 (42.5)	8,600 (49.5)	8,680 (52)
70						6,960 (24.5)	7,170 (37.5)	7,340 (45.5)	7,430 (48.5)
75							6,080 (31)	6,290 (40.5)	6,390 (44.5)
80							5,130 (23)	5,380 (35.5)	5,490 (40)
85								4,580 (29.5)	4,720 (35)
90								3,880 (22)	4,020 (29)
95									3,400 (21.5)
Minimum boom angle (°) for indicated length (no load)									0
Maximum boom length (ft.) at 0° boom angle (no load)									105

NOTE: ( ) Boom angles are in degrees.

#LMI operating code. Refer to LMI manual for operating instructions.

\*This capacity is based on maximum boom angle.

Lifting Capacities at Zero Degree Boom Angle On Outriggers Fully Extended - 360°									
Boom Angle	Main Boom Length in Feet								
	33	40	50	60	70	80	90	100	
0°	16,250 (28.2)	12,500 (35)	8,780 (45)	6,290 (55)	4,510 (65)	3,160 (75)	2,110 (85)	1,260 (95)	

NOTE: ( ) Reference radii in feet.

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## RT600E SERIES ON RUBBER CAPACITIES

STATIONARY CAPACITIES  
360°

Radius in Feet	#9005				
	Main Boom Length in Feet				
	33	40	50	60	70
10	38,550 (69.5)	38,550 (73.5)			
12	32,550 (65.5)	32,550 (70.5)	32,550 (74.5)		
15	23,700 (59.5)	23,700 (65.5)	23,700 (71)	23,700 (75.5)	
20	14,450 (47.5)	14,450 (57)	14,450 (64.5)	14,450 (70)	14,450 (73.5)
25	9,640 (32)	9,640 (47)	9,640 (58)	9,640 (65)	9,640 (69.5)
30		6,840 (34.5)	6,840 (50)	6,840 (59)	6,840 (64.5)
35			4,850 (41.5)	4,850 (53)	4,850 (60)
40			3,450 (30.5)	3,450 (46.5)	3,450 (54.5)
45				2,410 (38.5)	2,410 (49)
50				1,610 (28.5)	1,610 (43)
Min. boom angle (°) for indicated length (no load)					30
Max. boom length (ft.) at 0° boom angle (no load)					60

NOTE: ( ) Boom angles are in degrees.

#LMI operating code. Refer to LMI manual for operating instructions.

Lifting Capacities at Zero Degree Boom Angle  
On Rubber - 360°

Boom Angle	Main Boom Length in Feet				
	33	40	50		
0°	7,580 (28.2)	4,850 (35)	2,410 (45)		

NOTE: ( ) Reference radii in feet.

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STATIONARY CAPACITIES  
DEFINED ARC OVER FRONT (See Note 3)

Radius in Feet	#9005				
	Main Boom Length in Feet				
	33	40	50	60	70
10	46,600 (69.5)	40,800 (73.5)	34,600 (77)		
12	40,800 (65.5)	40,800 (70.5)	34,600 (74.5)		
15	34,000 (59.5)	34,000 (65.5)	34,000 (71)	26,650 (75.5)	21,500 (78)
20	26,050 (47.5)	26,050 (57)	26,050 (64.5)	26,050 (70)	21,500 (73.5)
25	18,200 (32)	18,200 (47)	18,200 (58)	18,200 (65)	18,200 (69.5)
30		13,100 (34.5)	13,100 (50)	13,100 (59)	13,100 (64.5)
35			10,050 (41.5)	10,050 (53)	10,050 (60)
40			7,900 (30.5)	7,900 (46.5)	7,900 (54.5)
45				6,290 (38.5)	6,290 (49)
50				5,050 (28.5)	5,050 (43)
55					4,060 (35.5)
60					3,260 (26.5)
Min. boom angle (°) for indicated length (no load)					0
Max. boom length (ft.) at 0° boom angle (no load)					70

NOTE: ( ) Boom angles are in degrees.

#LMI operating code. Refer to LMI manual for operating instructions.

Lifting Capacities at Zero Degree Boom Angle  
On Rubber - Defined Arc Over Front

Boom Angle	Main Boom Length in Feet				
	33	40	50	60	70
0°	14,550 (28.2)	10,050 (35)	6,290 (45)	4,060 (55)	2,590 (65)

NOTE: ( ) Reference radii in feet.

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ABQ08

# CALCULATION COVER SHEET



<b>Project:</b>	INEEL V-Tank Remediation Project				<b>Number of Sheets:</b> 1 of 8
<b>Site:</b>	INEEL Test Area North, Idaho Falls, Idaho				
<b>Calculation Number:</b>	ABQ08 – CE004	<b>Work Order Number:</b>	12393.002.001		
<b>Subject:</b>	Drum Storage/Water Storage/Decontamination Area Secondary Containment Requirements				
<b>Rev #:</b>	<b>Date:</b>	<b>Revision:</b>	<b>Calculated by:</b>	<b>Checked by:</b>	<b>Approved:</b>
RAA		60%	R. Ederer		
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RAD	9/27/01	Draft Final	D. Brennecke	B. Keshian	Jim Lockhart
RAE	10/23/01	Draft Final Polish	D. Brennecke	B. Keshian	Jim Lockhart <i>[Signature]</i> 10/24/01

CLIENT/SUBJECT \_\_\_\_\_ W.O. NO. \_\_\_\_\_

TASK DESCRIPTION \_\_\_\_\_ TASK NO. \_\_\_\_\_

PREPARED BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

APPROVED BY

DEPT \_\_\_\_\_ DATE \_\_\_\_\_

## Problem Statement:

Estimate the containment <sup>volume</sup> ~~area~~ required for  
~~storage~~ of contaminated liquids.  
secondary containment

## Method of Solution:

1. Determine the area needed for containers.
2. Determine the volume need for 35 yr / 24 hr storm.
3. Determine volume of liquids in containers.
4. Design containment berm with a capacity to contain the volume of contaminated liquids & 25 yr / 24 hr. storm.

## Sources of Information:

Design Drawings  
Design information from Bartlett, INEEL and  
BBU

NOAA ATLAS 2, VOL V, ISOPLUVIALS OF 25 yr / 24 hr.  
Precipitation in tenths of AN INCH  
TR-55

## Assumptions:

$$CN = 79$$

$$S = \frac{1000}{CN} - 10$$

$$Q = \frac{(P - 0.25)^2}{(P + 0.85)}$$

**DETERMINATION OF WATER SOURCES**

Item	Value	Units
Sources of water		
V-1	1164	gallons
V-2	1076	gallons
V-3	7648	gallons
V-9	70	gallons
Decon	2000	gallons
Runoff	15000	gallons
Rinse	500	gallons
Total water volume	27458	gallons

CLIENT/SUBJECT \_\_\_\_\_ W.O. NO. \_\_\_\_\_

TASK DESCRIPTION \_\_\_\_\_ TASK NO. \_\_\_\_\_

PREPARED BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

APPROVED BY	
DEPT _____	DATE _____

## Assumptions. Con't

- Contain 100% volume of the largest container or 10% of total storage volume plus volume of 25 YR 24 HR storm per 40 CFR 264.175b3

- Total storage volume:

Decon, Runoff, Rinse, Tank water  $\approx$  30,000 GALLONS

- Largest container will be 10,000 GAL water tank  
(12'  $\phi$  x 14' HIGH) Area req'd =  $\pi L^2 = 113$  SF/TANK

- ESTIMATED AREA OF CONTAINMENT AREA FROM DRAWINGS

$$\approx 6,000 \text{ SF} \times \frac{\text{mi}^2}{(5280 \text{ FT/m})^2} = 0.000215 \text{ mi}^2$$

CLIENT/SUBJECT \_\_\_\_\_ W.O. NO. \_\_\_\_\_

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PREPARED BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

APPROVED BY	
DEPT _____	DATE _____

Calculations:

Volume of Stormwater from within containment area

$$V = Q \times A_m \times 53.33$$

AC FT

$$A_m = \text{Area in mi}^2$$

$$Q = \frac{(P_{25} - 0.25)^2}{(P + 0.85)}$$

$$S = \frac{1000}{CN} - 10 \Rightarrow S = \frac{1000}{79} - 10 = 2.66$$

$$P_{25} = 2.0$$

$$Q = 0.522 \text{ in}$$

$$V = 0.522 \times 0.000215 \text{ mi}^2 \times 53.33$$

$$= 0.005985 \text{ AC-FT} \times \frac{43,560 \text{ SF}}{\text{AC}} = 260.7 \text{ CF}$$

$$= 260.7 \text{ CF} \times \frac{7.48 \text{ ga}}{\text{CF}}$$

$$= \underline{\underline{1950 \text{ GAL}}} \leftarrow$$

Storage Volume Req'd

$$\begin{aligned} \text{Vol} &= \text{Tank} + 25 \text{ YR } 24 \text{ HR STORM} \\ &= 10,000 \text{ GAL} + 1950 \text{ GAL} \end{aligned}$$

$$= \underline{\underline{11,950 \text{ GAL}}} \leftarrow$$

CLIENT/SUBJECT WHEEL V-TANK

W.O. NO. \_\_\_\_\_

TASK DESCRIPTION RECALCULATE SIZE OF DRUM STORAGE AREA TASK NO. \_\_\_\_\_PREPARED BY DFB DEPT \_\_\_\_\_ DATE 8/14/01  
10/22/01

APPROVED BY \_\_\_\_\_

MATH CHECK BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

METHOD REV. BY \_\_\_\_\_ DEPT \_\_\_\_\_ DATE \_\_\_\_\_

DEPT \_\_\_\_\_ DATE \_\_\_\_\_

ASSUMPTIONS

- DRUMS ARE MOVED DIRECTLY TO INTERIM STORAGE AFTER FILLING AND NO MORE THAN 10 DRUMS ARE STORED IN DRUM STORAGE AREA AT ONE TIME
- DRUM STORAGE AREA IS 25 <sup>sq</sup>/DRUM
- 10,000 GAL H<sub>2</sub>O STORAGE TANKS (12'8") REQUIRE 77 <sup>sq</sup> = 113 <sup>sq</sup>/TANK
- 30% ADDITIONAL AREA REQD FOR LOGISTICS
- 10,975 GAL OF SECONDARY CONTAINMENT STORAGE IS REQD (AS908 RAC)  
(10,000 GAL + 1950 = 10,975) ASSUMES 3,000 <sup>sq</sup> AREA CATCHES STORM WATER

CALCULATE AREA REQD

DRUMS	25 <sup>sq</sup> /DRUM x 10 DRUMS	= 250 <sup>sq</sup>
10,000 GAL WATER STORAGE	113 <sup>sq</sup> /TANK x 8 TANKS	= 904 <sup>sq</sup>
1,000 GAL WATER CONTAINER (6' x 14'5" = 87 <sup>sq</sup> )	87 <sup>sq</sup> /TANK x 10 TANKS	= 870 <sup>sq</sup>
		589 <sup>sq</sup> 1346
	+30% LOGISTICS	771 <sup>sq</sup> 404
		768 <sup>sq</sup> 1750
	ACCESS ROAD 12' x 6'	72 <sup>sq</sup>
		1486 <sup>sq</sup> 280 <sup>sq</sup>
		2470 <sup>sq</sup> 1500 <sup>sq</sup> ←

CALCULATE MIN AREA REQD ASSUMING MINIMUM STORAGE DEPTH OF 1 FOOT

$$10,975 \text{ GAL} \times \frac{\text{ft}^3}{7.48 \text{ GAL}} \times \frac{1}{1 \text{ ft}} = 1467 \text{ ft}^2 \text{ MIN AREA REQD}$$

$$\Rightarrow \text{MINIMUM DIMENSIONS OF AREA} = \sqrt{1500 \text{ ft}^2} = 38.7'$$

∴ MAKE AREA 50' x 60' TO ACCOMMODATE  
SEMI TRACTOR TRAILER DECOR

